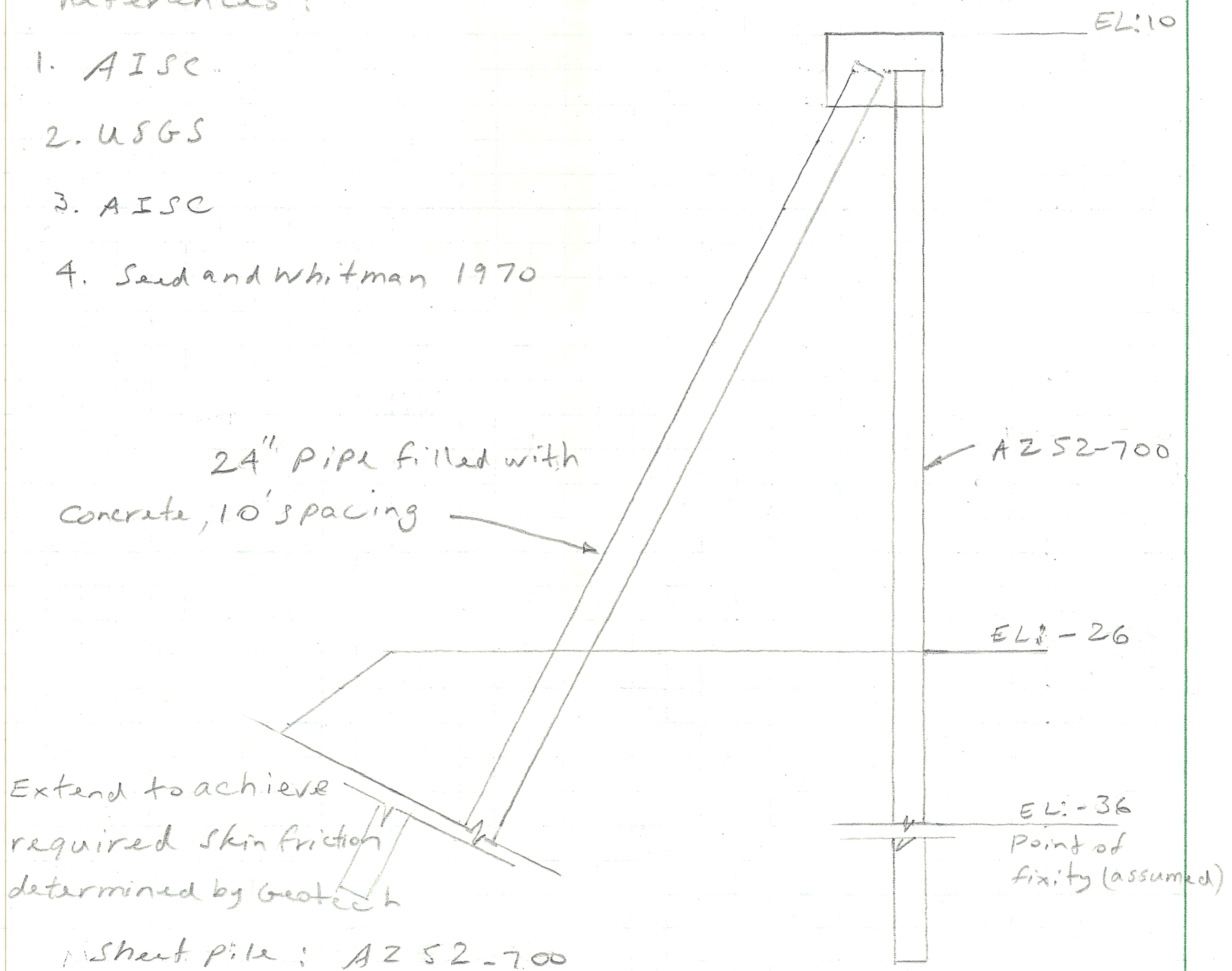


Sheet pile with batter pile Design

References:

1. AISC
2. USGS
3. AISC
4. Seed and Whitman 1970



weight: 50.93 lbs/ft²

$S = 95.9 \text{ in}^3/\text{ft}$ Section Modulus

$I = 953 \text{ in}^4/\text{ft}$ Moment of Inertia

Depth: 19.88 in

Batter Pile: 24" ϕ with 10' spacing, filled with concrete

Assumptions: Water ballanced on both side of sheet pile
No water pressure

$K_a = .35$ active pressure coefficient

$K_p = 3.0$ Passive Pressure Coefficient

$\gamma_s = 120 \text{ lbs/ft}^3$ Soil unit weight

$\gamma_{sub} = 57.6 \text{ lbs/ft}^3$ submerged soil unit weight

Point of pile fixity = EL: -3.6

Surcharge: 250 PSF.

F_a , Active Pressure: $w_a = K_a \gamma_{sub} H = .35 \times 57.6 \times 46 = 927.36 \text{ lbs/ft}^2$

F_p , Passive Pressure: $w_p = K_p \gamma_{sub} H = 3 \times 57.6 \times 10 = 1728 \text{ lbs/ft}^2$

F_s , Surcharge charge: $w_{surcharge} = K_a \times 250 = 87.50 \text{ lbs/ft}^2$

Concrete cap weight: $5' \times 7' \times 10' \times 150 = 52,500 \text{ lbs}$

Seismic:

Latitude: 37.791297, Longitude: -122.287034

USGS 2014:

Return period of 975 years: ER1110-2-1806

$PGA = .6188g$, $S_s = 1.441g$, $S_1 = .4535$

$w_s = \frac{3}{4} K_h \gamma H$ Seed and Whitman 1970

$$K_h = \frac{S_{Ds}}{2.5} = \frac{\frac{2}{3} \times S_1}{2.5} = \frac{\frac{2}{3} \times 1.441}{2.5} = .384$$

E, $w_s = \frac{3}{4} \times .384 \times 120 \times 46 = 1590 \text{ lbs/ft}^2$

STAAD output:

Load combination: 1.0 D + 1.0 F_a + 1.0 F_p + 1.0 F_s + 0.7 E
A

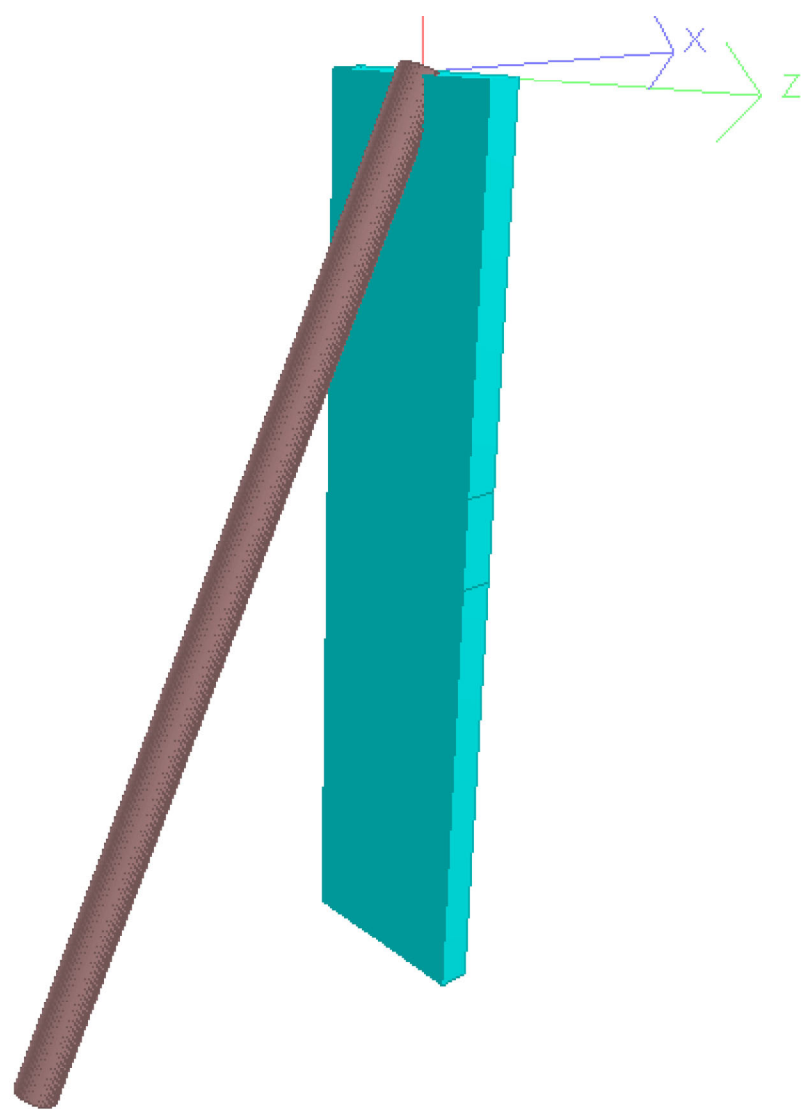
Max. Moment @ point of fixity = 2,855 Kip-ft

Sheet pile Moment Capacity:

$F_y = 60,000 \text{ PSI}$, $S = 95.9 \text{ in}^3/\text{ft}$

$M_n = F_y \times S = 60,000 \times 95.9 \times 10 = 57,540 \text{ K-in}$
 $= 4,795 \text{ K-ft}$

$\frac{M_n}{\phi_b} = \frac{4,795}{1.67} = 2,871 \text{ K-ft} > 2,855 \text{ K-ft}$ okay



PAGE NO. 1

```

*****
*
*          STAAD.Pro CONNECT Edition          *
*          Version  21.00.02.30              *
*          Proprietary Program of            *
*          Bentley Systems, Inc.             *
*          Date=    OCT 12, 2021              *
*          Time=    14:44:19                 *
*
*  Licensed to: USACE HQ                     *
*****

```

1. STAAD SPACE

INPUT FILE: C:\Users\g2ecdmr9\Desktop\Oakland Basin Bulkhead\STAAD\Oakland Basin Sheet Pile 10-12-21-3.STD

2. START JOB INFORMATION

3. ENGINEER DATE 05-OCT-21

4. END JOB INFORMATION

5. INPUT WIDTH 79

6. UNIT FEET KIP

7. JOINT COORDINATES

8. 1 0 0 0; 2 0 -46 0; 3 -23 -45 0; 4 0 -36 0

9. MEMBER INCIDENCES

10. 2 1 3; 3 1 4; 4 4 2

11. START USER TABLE

12. TABLE 1

13. UNIT FEET KIP

14. PRISMATIC

15. SHEETPILE

16. 1.04 0.422 4 4 1.04 1.04 1.65 10

17. TABLE 2

18. UNIT FEET KIP

19. PRISMATIC

20. SHEETPILE2

21. 1.04 0.46 4 4 1.04 1.04 1.66 10

22. TABLE 3

23. UNIT FEET KIP

24. PRISMATIC

25. SHEETPILE2

26. 1.04 0.46 4 4 1.04 1.04 1.66 10

27. END

28. DEFINE MATERIAL START

29. ISOTROPIC STEEL

30. E 4.176E+06

31. POISSON 0.3

32. DENSITY 0.489024

33. ALPHA 6.5E-06

34. DAMP 0.03

35. TYPE STEEL

36. STRENGTH FY 5184 FU 8352 RY 1.5 RT 1.2

37. END DEFINE MATERIAL

38. MEMBER PROPERTY AMERICAN

STAAD SPACE

-- PAGE NO. 2

```

39. 2 TABLE ST PIPS240
40. MEMBER PROPERTY
41. 3 4 UPTABLE 2 SHEETPILE2
42. CONSTANTS
43. MATERIAL STEEL ALL
44. SUPPORTS
45. 2 3 FIXED
46. MEMBER RELEASE
47. 2 START MZ
48. LOAD 1 LOADTYPE NONE TITLE SELF WEIGHT
49. SELFWEIGHT Y -1 LIST 2 3
50. LOAD 2 LOADTYPE NONE TITLE ACTIVE SOIL PRESSURE
51. MEMBER LOAD
52. 4 TRAP GX -7.25478 -9.27 0 10
53. 3 TRAP GX 0 -7.25478 0 36
54. LOAD 3 LOADTYPE NONE TITLE SESIMIC PRESSURE
55. MEMBER LOAD
56. 4 TRAP GX -3.45652 0 0 10
57. 3 TRAP GX -15.9 -3.45652 0 36
58. LOAD 4 LOADTYPE NONE TITLE CONCRETE CAP
59. JOINT LOAD
60. 1 FY -52.5
61. LOAD 5 LOADTYPE NONE TITLE PASSIVE PRESSURE
62. MEMBER LOAD
63. 4 TRAP GX 0 17.28 0 10
64. LOAD 6 LOADTYPE NONE TITLE SURCHARGE
65. MEMBER LOAD
66. 4 UNI GX -0.88 0 10
67. 3 UNI GX -0.88 0 36
68. LOAD COMB 7 DEAD + SOIL + SURCHARGE
69. 1 1.0 2 1.0 4 1.0 5 1.0 6 1.0
70. LOAD COMB 8 DEAD + SOIL + SURCHARGE + .7 SEISMIC
71. 1 1.0 2 1.0 3 0.7 4 1.0 5 1.0 6 1.0
72. PERFORM ANALYSIS

```

P R O B L E M S T A T I S T I C S

NUMBER OF JOINTS	4	NUMBER OF MEMBERS	3
NUMBER OF PLATES	0	NUMBER OF SOLIDS	0
NUMBER OF SURFACES	0	NUMBER OF SUPPORTS	2

Using 64-bit analysis engine.

SOLVER USED IS THE OUT-OF-CORE BASIC SOLVER

```

ORIGINAL/FINAL BAND-WIDTH= 3/ 3/ 12 DOF
TOTAL PRIMARY LOAD CASES = 6, TOTAL DEGREES OF FREEDOM = 12
TOTAL LOAD COMBINATION CASES = 2 SO FAR.
SIZE OF STIFFNESS MATRIX = 1 DOUBLE KILO-WORDS
REQRD/AVAIL. DISK SPACE = 12.0/ 769126.4 MB

```

STAAD SPACE

-- PAGE NO. 3

*WARNING- APPLIED SELFWEIGHT IS LESS THAN TOTAL WEIGHT OF ALL
STRUCTURAL ELEMENTS IN LOAD CASE 1 ALONG Y.
THIS COULD BE DUE TO SELFWEIGHT APPLIED TO SPECIFIC
LIST OF MEMBERS/PLATES/SOLIDS/SURFACES.

TOTAL UNFACTORED WEIGHT OF THE STRUCTURE =	27.857 KIP
TOTAL UNFACTORED WEIGHT OF THE STRUCTURE APPLIED =	22.771 KIP

73. PARAMETER 1
74. CODE LRFD
75. LOAD LIST 7 8
76. CHECK CODE ALL

STEEL DESIGN

STAAD SPACE

-- PAGE NO. 4

STAAD.Pro CODE CHECKING - (LRFD 3RD EDITION) v1.0

ALL UNITS ARE - KIP FEET (UNLESS OTHERWISE Noted)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
=====					

2	ST	PIPS240	(AISC SECTIONS)		
		PASS	LRFD-H1-1A-C	0.741	8
		425.62 C	0.00	-20.90	33.69

****WARNING-THIS VERSION DOES NOT DESIGN PRISMATIC SECTIONS (MEMBER 3).**
 PLEASE DEFINE THE PROPERTIES THROUGH A USER TABLE GENERAL SECTION.

****WARNING-THIS VERSION DOES NOT DESIGN PRISMATIC SECTIONS (MEMBER 4).**
 PLEASE DEFINE THE PROPERTIES THROUGH A USER TABLE GENERAL SECTION.

***** END OF TABULATED RESULT OF DESIGN *****

77. PRINT MEMBER FORCES
 MEMBER FORCES

STAAD SPACE

-- PAGE NO. 5

MEMBER END FORCES STRUCTURE TYPE = SPACE

ALL UNITS ARE -- KIP FEET (LOCAL)

MEMBER	LOAD	JT	AXIAL	SHEAR-Y	SHEAR-Z	TORSION	MOM-Y	MOM-Z
2	7	1	120.75	0.91	0.00	0.00	0.00	0.00
		3	-124.73	1.12	0.00	0.00	0.00	-5.45
	8	1	422.97	1.30	0.00	0.00	0.00	0.00
		3	-426.95	0.73	0.00	0.00	0.00	14.25
3	7	1	-54.61	55.77	0.00	0.00	0.00	0.00
		4	36.30	106.50	0.00	0.00	0.00	-129.73
	8	1	-323.54	193.65	0.00	0.00	0.00	0.00
		4	305.23	212.50	0.00	0.00	0.00	-496.49
4	7	4	-36.30	-106.50	0.00	0.00	0.00	129.73
		2	36.30	111.52	0.00	0.00	0.00	-1347.06
	8	4	-305.23	-212.50	0.00	0.00	0.00	496.49
		2	305.23	229.63	0.00	0.00	0.00	-2854.50

***** END OF LATEST ANALYSIS RESULT *****

78. PRINT SUPPORT REACTION

SUPPORT REACTION

STAAD SPACE

-- PAGE NO. 6

SUPPORT REACTIONS -UNIT KIP FEET STRUCTURE TYPE = SPACE

JOINT	LOAD	FORCE-X	FORCE-Y	FORCE-Z	MOM-X	MOM-Y	MOM Z
2	7	111.52	-36.30	0.00	0.00	0.00	-1347.06
	8	229.63	-305.23	0.00	0.00	0.00	-2854.50
3	7	55.77	111.57	0.00	0.00	0.00	5.45
	8	193.65	380.50	0.00	0.00	0.00	-14.25

***** END OF LATEST ANALYSIS RESULT *****

79. FINISH

***** END OF THE STAAD.Pro RUN *****

**** DATE= OCT 12,2021 TIME= 14:44:19 ****

STAAD SPACE

-- PAGE NO. 7

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*****
*   For technical assistance on STAAD.Pro, please visit   *
*   http://www.bentley.com/en/support/                   *
*                                                         *
*   Details about additional assistance from               *
*   Bentley and Partners can be found at program menu    *
*   Help->Technical Support                               *
*                                                         *
*   Copyright (c) 1997-2017 Bentley Systems, Inc.        *
*   http://www.bentley.com                               *
*****
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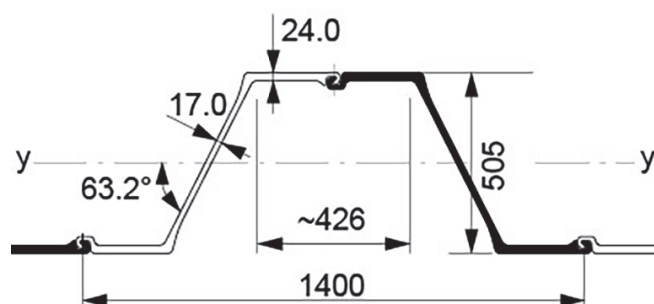


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AZ 52-700 (Hot rolled sheet piles)



Section description	Product group	Shape	Section Modulus	Moment of Inertia	Width	Height	Thickness flange	Thickness web	Weight single	Weight	Coating 2 sides	Coating area
			in ³ /ft cm ³ /m	in ⁴ /ft cm ⁴ /m	inch mm	inch mm	inch mm	inch mm	lbs/ft kg/m	lbs/ft ² kg/m ²	ft ² /ft m ² /m	ft ² /ft m ² /m
AZ 52-700	Hot rolled sheet piles	Z	95.9	953.0	27.56	19.88	0.945	0.669	117.0	50.93	6.70	1.46
			5,156	130,140	700	505	24.0	17.0	174.05	248.66	2.05	1.46

Production acc. EN10248 1&2 in various steel grades
Origin: Luxembourg
Datasheet for estimation and comparison purposes

✓ PILING PRODUCTS

✓ SHEET PILES

✓ PIPES

✓ H-BEAMS